

## Lecture 27: Linear Elasticity

Logistics: - please fill out course evaluations

Last time: Hyperelastic materials

$$\hat{\underline{\underline{P}}}(\underline{\underline{F}}) = \underline{\underline{D}}W(\underline{\underline{F}})$$

Material frame indiff

$$\hat{\underline{\underline{P}}}(\underline{\underline{F}}) = 2 \underline{\underline{F}} \underline{\underline{D}}\bar{W}(\underline{\underline{C}})$$

⇒ inherently nonlinear

- Mechanical energy inequality (2<sup>nd</sup> law)  
closed system.

energetically passive

- Common Model

• Neo-Hookean

• Mooney-Rivlin

• Ogden

Today: → linearize elastodynamic eqns

# Linear Elasticity

initial boundary value problem

$$\text{PDE: } \rho_0 \ddot{\underline{\varphi}} = \nabla_x \cdot \hat{\underline{\underline{P}}}(\underline{\underline{F}}) + \rho_0 \underline{\underline{b}}_m \quad x \in \Omega \times [0, T]$$

$$\text{BC: } \underline{\underline{\varphi}} = \underline{\underline{g}} \quad \partial\Omega_d$$

$$\hat{\underline{\underline{P}}}(\underline{\underline{F}}) \underline{\underline{N}} = \underline{\underline{h}} \quad \partial\Omega_\delta$$

$$\text{IC: } \underline{\underline{\varphi}} = \underline{\underline{X}} \quad \Omega$$

$$\dot{\underline{\underline{\varphi}}} = \underline{\underline{V}}_0 \quad \Omega$$

Considers a stress free initial condition

$$\text{at } t=0 \quad \underline{\underline{F}} = \underline{\underline{I}} \Rightarrow \hat{\underline{\underline{P}}}(\underline{\underline{I}}) = \hat{\underline{\underline{\Sigma}}}(\underline{\underline{I}}) - \hat{\underline{\underline{\delta}}}(\underline{\underline{I}}) = \underline{\underline{0}}$$

If forcings are small

$$|\underline{\underline{b}}_m| = O(\epsilon) \quad |g - \underline{\underline{X}}| = O(\epsilon) \quad |\underline{\underline{h}}| = O(\epsilon) \quad |\underline{\underline{V}}_0| = O(\epsilon)$$

where  $0 \leq \epsilon \ll 1$

we expect that the displacement is small

$$|u(x, t)| = |\varphi(x, t) - \underline{\underline{X}}| = O(\epsilon)$$

$\Rightarrow$  assuming well-posed system

## Linearized equations

Express forcings

$$\underline{b}_m^\epsilon = \epsilon \underline{b}_m \quad \underline{g}^\epsilon = \underline{\chi} - \epsilon \underline{g} \quad \underline{h}^\epsilon = \epsilon \underline{h} \quad \underline{V}_0^\epsilon = \epsilon \underline{V}_0$$

then  $\varphi^\epsilon = \underline{\chi} + \epsilon \underline{u} + \mathcal{O}(\epsilon^2)$   $\underline{u}^\epsilon = \epsilon \underline{u} = \varphi^\epsilon - \underline{\chi}$

def  
disp. grad.:  $\underline{F}^\epsilon = \nabla_x \varphi^\epsilon = \underline{\mathbb{I}} + \epsilon \nabla_x \underline{u}$

substitute into PDE

$$\rho_0 \frac{\partial^2}{\partial t^2} (\underline{\chi} + \underline{u}^\epsilon) = \nabla_x \cdot [\hat{\underline{P}}(\underline{F}^\epsilon)] + \rho_0 \underline{b}_m^\epsilon$$

$$\rho_0 \underline{\ddot{u}}^\epsilon = \nabla_x \cdot [\hat{\underline{P}}(\underline{F}^\epsilon)] + \rho_0 \underline{b}_m^\epsilon$$

Need to deal with  $\nabla_x \cdot \hat{\underline{P}}(\underline{F}^\epsilon)$

Introduce 3 4<sup>th</sup> order tensors:

$$A_{ijkl} = \frac{\partial \hat{P}_{ij}}{\partial F_{kl}}(\underline{\mathbb{I}}) \quad B_{ijkl} = \frac{\partial \hat{\Sigma}_{ij}}{\partial F_{kl}}(\underline{\mathbb{I}}) \quad C_{ijkl} = \frac{\partial \hat{\delta}_{ij}}{\partial F_{kl}}(\underline{\mathbb{I}})$$

in tensor notation  $\underline{H} = \nabla_x \underline{u}$

$$\underline{A} \underline{H} = \frac{d}{d\epsilon} \hat{\underline{P}}(\underline{\mathbb{I}} + \epsilon \underline{H})|_{\epsilon=0} = D \hat{\underline{P}}(\underline{\mathbb{I}}) \underline{H} \quad \underline{A} = D \hat{\underline{P}}(\underline{\mathbb{I}})$$

$$\underline{B} \underline{H} = D \hat{\underline{\Sigma}}(\underline{\mathbb{I}}) \underline{H}$$

$$\underline{C} \underline{H} = D \hat{\underline{\delta}}(\underline{\mathbb{I}}) \underline{H}$$

Express stress response in Taylor series

$$\hat{\underline{\underline{P}}}(\underline{\underline{E}}^\epsilon) = \hat{\underline{\underline{P}}}(\underline{\underline{I}} + \epsilon \underline{\underline{H}}) = \hat{\underline{\underline{P}}}(\underline{\underline{I}}) + \epsilon \underline{\underline{A}} \underline{\underline{H}} + O(\epsilon^2)$$

$$= \epsilon \underline{\underline{A}} \underline{\underline{H}}$$

substitute into lin. mom. bal.  $\underline{\underline{u}}^\epsilon = \epsilon \underline{\underline{u}}$   $\underline{\underline{b}}_m^\epsilon = \epsilon \underline{\underline{b}}_m$

$$\rho_0 \epsilon \underline{\underline{\ddot{u}}} = \epsilon \nabla_x \cdot [\underline{\underline{A}} \nabla \underline{\underline{u}}] + \rho_0 \epsilon \underline{\underline{b}}_m$$

linearized <sup>lin.</sup> mom. balance

$$\rho_0 \underline{\underline{\ddot{u}}} = \nabla_x \cdot [\underline{\underline{A}} \nabla \underline{\underline{u}}] + \rho_0 \underline{\underline{b}}_m$$

lin. elasto dyn. equations

$$\nabla_x \cdot \hat{\underline{\underline{P}}}(\underline{\underline{F}})$$

$$F \hat{\underline{\underline{P}}}(\underline{\underline{C}})$$

In lin. case,  $|\varphi - \underline{\underline{x}}| = O(\epsilon)$   $\underline{\underline{x}} \sim X$

⇒ don't need to distinguish material & current reference frames.

if  $\varphi \underline{\underline{\ddot{u}}} = 0 \Rightarrow$  elastic static eqns

$$\nabla \cdot [\underline{\underline{A}} \nabla \underline{\underline{u}}] + \rho_0 \underline{\underline{b}}_m = 0$$

# Elasticity Tensors

introduced 4<sup>th</sup> tensors:

$$\underline{A} = D \hat{\underline{P}}(\underline{I}) \quad \underline{B} = D \hat{\underline{Z}}(\underline{I}) \quad \underline{C} = D \hat{\underline{S}}(\underline{I})$$

If initial cond. is stress free  $\Rightarrow \underline{A} = \underline{B} = \underline{C}$

Example:  $\underline{A} = \underline{C}$

$$\hat{\underline{P}}(\underline{F}) = \det(\underline{F}) \hat{\underline{S}}(\underline{F}) \underline{F}^{-T}$$

differentiate both sides at  $\underline{I}$

$$\begin{aligned} D \hat{\underline{P}}(\underline{I}) \underline{H} &= \underline{A} \underline{H} = \frac{d}{d\varepsilon} \left[ \det(\underline{I} + \varepsilon \underline{H}) \hat{\underline{S}}(\underline{I} + \varepsilon \underline{H}) (\underline{I} + \varepsilon \underline{H})^{-T} \right] \Big|_{\varepsilon=0} \\ &= \frac{d}{d\varepsilon} [\det(\underline{I} + \varepsilon \underline{H})] \Big|_{\varepsilon=0} \hat{\underline{S}}(\underline{I}) \underline{I}^{-T} \\ &\quad + \det(\underline{I}) \frac{d}{d\varepsilon} [\hat{\underline{S}}(\underline{I} + \varepsilon \underline{H})] \Big|_{\varepsilon=0} \underline{I}^{-T} \\ &\quad + \det(\underline{I}) \hat{\underline{S}}(\underline{I}) \frac{d}{d\varepsilon} (\underline{I} + \varepsilon \underline{H})^{-T} \end{aligned}$$

where  $\det(\underline{I}) = 1$  and stress free IC  $\hat{\underline{S}}(\underline{I}) = 0$

$$\begin{aligned} D \hat{\underline{P}}(\underline{I}) \underline{H} &= \underline{A} \underline{H} = \frac{d}{d\varepsilon} \hat{\underline{S}}(\underline{I} + \varepsilon \underline{H}) \Big|_{\varepsilon=0} = D \hat{\underline{S}}(\underline{I}) \underline{H} = \underline{C} \underline{H} \\ &\Rightarrow \underline{A} = \underline{C} \end{aligned}$$

$\Rightarrow$  elastic solid with stress free IC has a unique elasticity tensor  $\underline{C}$  which can be determined

from any stress response function  $\hat{\underline{\underline{P}}}(\underline{\underline{F}}) \hat{\underline{\underline{\Sigma}}}(\underline{\underline{F}}) \hat{\underline{\underline{\delta}}}(\underline{\underline{F}})$   
 by linearizing around  $\underline{\underline{I}}$ .

### Balance of angular momentum

$$\hat{\underline{\underline{\delta}}}(\underline{\underline{F}})^T = \hat{\underline{\underline{\delta}}}(\underline{\underline{F}})$$

$$\begin{aligned} [\underline{\underline{C}}]^{TT} &= \left( \frac{d}{d\varepsilon} \hat{\underline{\underline{\delta}}}(\underline{\underline{I}} + \varepsilon \underline{\underline{H}}) \Big|_{\varepsilon=0} \right)^T \\ &= \frac{d}{d\varepsilon} \hat{\underline{\underline{\delta}}}(\underline{\underline{I}} + \varepsilon \underline{\underline{H}})^T \Big|_{\varepsilon=0} \quad \text{note } \det(\underline{\underline{I}} + \varepsilon \underline{\underline{H}}) \approx 1 \\ &= \frac{d}{d\varepsilon} \hat{\underline{\underline{\delta}}}(\underline{\underline{I}} + \varepsilon \underline{\underline{H}}) \Big|_{\varepsilon=0} = \underline{\underline{C}} \end{aligned}$$

Implies that  $\underline{\underline{C}}$  has left minor symmetry

$$C_{ijkl} = C_{jikl} \quad \text{or} \quad \underline{\underline{A}} : \underline{\underline{C}} \underline{\underline{B}} = \text{sym}(\underline{\underline{A}}) : \underline{\underline{C}} \underline{\underline{B}}$$

Frame-indifference for isotropic solid

$$\hat{\underline{\underline{\delta}}}(\underline{\underline{Q}} \underline{\underline{F}}) = \underline{\underline{Q}} \hat{\underline{\underline{\delta}}}(\underline{\underline{F}}) \underline{\underline{Q}}^T \quad \text{taking } \underline{\underline{F}} = \underline{\underline{I}}, \hat{\underline{\underline{\delta}}}(\underline{\underline{I}}) = \underline{\underline{0}}$$

$$\hat{\underline{\underline{\delta}}}(\underline{\underline{Q}}) = \underline{\underline{0}}$$

Use the fact that any infinitesimal rotation

can be written as a matrix exponential  
of a skew tensor  $\underline{\underline{W}} = -\underline{\underline{W}}^T$

$$\exp(\underline{\underline{A}}) = \sum_{j=0}^{\infty} \frac{1}{j!} \underline{\underline{A}}^j = \underline{\underline{I}} + \underline{\underline{A}} + \frac{1}{2} \underline{\underline{A}}^2 + \dots$$

We write  $\underline{\underline{Q}} = \exp(\epsilon \underline{\underline{W}})$

$$\hat{\underline{\underline{Q}}}(\underline{\underline{Q}}) = \hat{\underline{\underline{Q}}}(\exp(\epsilon \underline{\underline{W}})) = \underline{\underline{Q}}$$

By definition

$$\begin{aligned} \mathbb{C} \underline{\underline{W}} &= D_{\underline{\underline{Q}}} \hat{\underline{\underline{Q}}}(\underline{\underline{I}}) \underline{\underline{W}} = \left. \frac{d}{d\epsilon} \hat{\underline{\underline{Q}}}(\underline{\underline{I}} + \epsilon \underline{\underline{W}}) \right|_{\epsilon=0} \\ &= \left. \frac{d}{d\epsilon} \hat{\underline{\underline{Q}}}(\underline{\underline{I}} + \epsilon \underline{\underline{W}} + \frac{1}{2} \epsilon^2 \underline{\underline{W}}^2 + \dots) \right|_{\epsilon=0} \\ &= \left. \frac{d}{d\epsilon} \hat{\underline{\underline{Q}}}(\exp(\epsilon \underline{\underline{W}})) \right|_{\epsilon=0} = \underline{\underline{Q}} \end{aligned}$$

$$\mathbb{C} \underline{\underline{W}} = \underline{\underline{Q}}$$

This implies

$$\mathbb{C} \underline{\underline{A}} = \mathbb{C} \text{sym}(\underline{\underline{A}}) + \mathbb{C} \text{skew}(\underline{\underline{A}})$$

$\Rightarrow \mathbb{C}$  has a right minor symmetry  $C_{ijkl} = C_{jilk}$

In summary:

- 1) Aug. min. bal. : left minor sym.
- 2) Frame. indiff : right minor sym.

## Elasticity tensors for isotropic solid

Let  $\underline{\underline{\sigma}}$  be frame indifferent Cauchy stress response for an elastic body with a stress free initial condition. If body is isotropic then  $\underline{\underline{C}}$  takes the form

$$\underline{\underline{C}} \underline{\underline{H}} = \lambda \operatorname{tr}(\underline{\underline{H}}) \underline{\underline{I}} + 2\mu \operatorname{sym}(\underline{\underline{H}}) \quad (\text{Lecture 21})$$

## Linearized Isotropic Elasticity

lin. mec. bal.

$$\rho_0 \underline{\underline{\ddot{u}}} = \nabla \cdot [\underline{\underline{C}} \nabla \underline{\underline{u}}] + \rho_0 \underline{\underline{b}}$$

with  $\underline{\underline{C}} \nabla \underline{\underline{u}} = \lambda \operatorname{tr}(\nabla \underline{\underline{u}}) \underline{\underline{I}} + 2\mu \operatorname{sym}(\nabla \underline{\underline{u}})$

need to evaluate  $\nabla \cdot \underline{\underline{C}} \nabla \underline{\underline{u}}$

$$\begin{aligned} \nabla \cdot [\underline{\underline{C}} \nabla \underline{\underline{u}}] &= (\lambda u_{k,k} \delta_{ij} + \mu u_{i,j} + \mu u_{j,i})_{,j} \underline{\underline{e}}_i \\ &= (\lambda u_{k,kj} \delta_{ij} + \mu u_{i,jj} + \mu u_{j,ij}) \underline{\underline{e}}_i \\ &= \lambda u_{k,ki} + \mu u_{i,jj} + \mu u_{j,ji} \\ &= \lambda \nabla(\nabla \cdot \underline{\underline{u}}) + \mu \nabla^2 \underline{\underline{u}} + \mu \nabla(\nabla \cdot \underline{\underline{u}}) \end{aligned}$$



subst. into lin. mom. bal.

$$\rho_0 \ddot{\underline{u}} = \mu \nabla^2 \underline{u} + (\lambda + \mu) \nabla (\nabla \cdot \underline{u}) + \rho_0 \underline{b}$$

Navier equation

General linear elastic solid

Stress response function:

$$\hat{\underline{P}}(\underline{E}) = \underline{C} \underline{e} \quad \underline{e} = \text{sym}(\nabla \underline{u})$$

Strain-energy density

$$W(\underline{E}) = \frac{1}{2} \underline{e} : \underline{C} \underline{e}$$

Isotropic model:

$$\underline{C} \underline{e} = \lambda \text{tr}(\underline{e}) \underline{\mathbb{I}} + 2\mu \underline{e}$$

The St. Venant-Kirchhoff model extends this lin. model to large deformation by replacing

the infinitesimal strain tensor  $\underline{e}$  with

Green Lagrange strain tensor  $\underline{E} = \frac{1}{2}(\underline{C} - \underline{\mathbb{I}})$

$$\text{Linear: } \hat{\underline{\underline{\Sigma}}} = \underline{\underline{C}} \underline{\underline{e}} = \lambda \text{tr}(\underline{\underline{e}}) + 2\mu \underline{\underline{e}} \quad \underline{\underline{e}} = \text{sym}(\nabla u)$$

$$\text{Non-linear: } \hat{\underline{\underline{\Sigma}}} = \underline{\underline{y}} = \lambda \text{tr}(\underline{\underline{E}}) + 2\mu \underline{\underline{E}} \quad \underline{\underline{E}} = \frac{1}{2}(\underline{\underline{C}} - \underline{\underline{I}})$$

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Thank you